

# R-KEX II with Rebars as an Anchor

Premium pure epoxy resin approved for use with reinforcement bars



## Approvals and Reports

- ETA-13/0454



## Product information

### Features and benefits

- The strongest resin in the epoxy resin class
- Approved for use with rebar as in anchor for use in non-cracked concrete (ETAG001 Option 7)
- Suitable for use in dry and wet substrates including under water
- Very high chemical resistance – suitable for applications exposed to influence of various agents (industrial or marine environment)
- Minimal shrinkage provides option of use in diamond-drilled holes and oversized holes
- Extended bonding time ensures easy installation of metal components (up to 30 min. in 20°C)
- For use in positive temperatures

### Applications

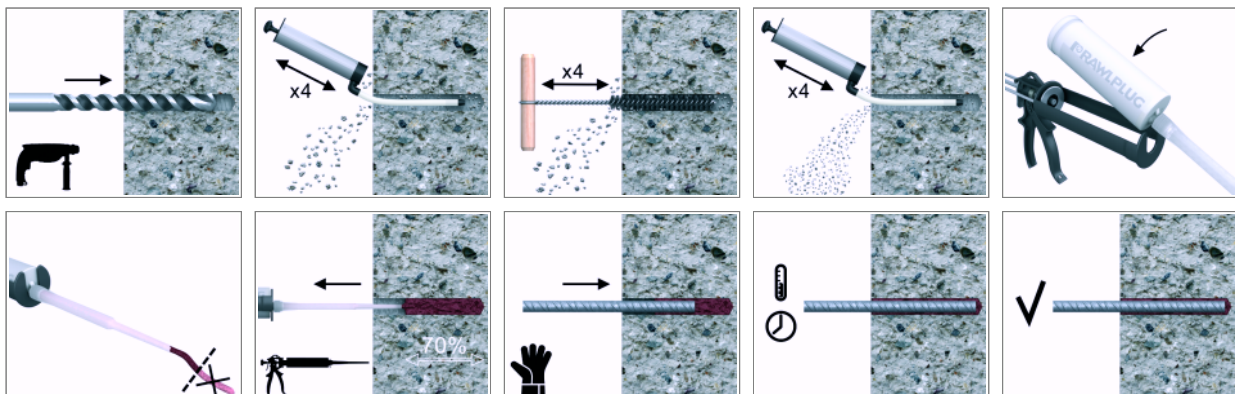
- Safety barriers
- Temporary works/formworks support systems
- Rebar
- Curtain walling
- Formwork support systems
- Masonry support
- Platforms
- Structural steelwork
- Rebar dowelling
- Starter bars
- Rebar missed-outs

### Base materials

#### Approved for use in:

- Non-cracked concrete C20/25-C50/60

## Installation guide



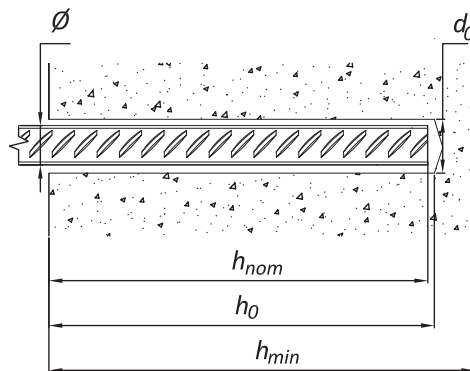
## Product information

1. Drill hole to the required diameter and depth for rebar size being used.
2. Clean the hole with brush and hand pump at least four times each. It is very important and necessary before installation.
3. Insert cartridge into gun and attach nozzle.
4. Dispense to waste until even colour is obtained.
5. Insert the mixing nozzle to the far end of the hole and inject resin, slowly withdrawing the nozzle as the hole is filled to 2/3 of its depth.
6. Immediately insert the rebar, slowly and with slight twisting motion. Remove any excess resin around the hole before it sets and leave it undisturbed until the curing time elapses.
7. Drill hole to the required diameter and depth for stud size being used.
8. Clean the hole with brush and hand pump at least four times each. It is very important and necessary before installation.
9. Insert cartridge into gun and attach nozzle.
10. Dispense to waste until even colour is obtained.
11. Insert the mixing nozzle to the far end of the hole and inject resin, slowly withdrawing the nozzle as the hole is filled to 2/3 of its depth.
12. Immediately insert the rebar, slowly and with slight twisting motion. Remove any excess resin around the hole before it sets and leave it undisturbed until the curing time elapses.

Size	Product Code	Resin	Description / Resin Type	Volume
				[m <sup>l</sup> ]
M8	R-KEX-II-385	R-KEX II	Epoxy Resin	385
M30	R-KEX-II-600			600

Product Code	Resin	Description / Resin Type	Volume
			[m <sup>l</sup> ]
R-KEX-II-385	R-KEX II	Epoxy Resin	385
R-KEX-II-600			600

## Installation data



### REBARS AS ANCHORS

Size			Ø8	Ø10	Ø12	Ø14	Ø16	Ø20	Ø25	Ø32
Thread diameter	d	[mm]	8	10	12	14	16	20	25	32
Hole diameter in substrate	d <sub>0</sub>	[mm]	12	14	18	18	22	26	32	40
Min. hole depth in substrate	h <sub>0</sub>	[mm]	h <sub>ef</sub> + 5	h <sub>ef</sub> + 5	h <sub>ef</sub> + 5	h <sub>ef</sub> + 5	h <sub>ef</sub> + 5	h <sub>ef</sub> + 5	h <sub>ef</sub> + 5	h <sub>ef</sub> + 5
<b>MINIMUM EMBEDMENT DEPTH</b>										
Installation depth	h <sub>nom, min</sub>	[mm]	60	70	80	80	100	120	140	165
<b>STANDARD EMBEDMENT DEPTH</b>										
Installation depth	h <sub>nom, s</sub>	[mm]	80	90	110	110	125	170	210	240
<b>MAXIMUM EMBEDMENT DEPTH</b>										
Installation depth	h <sub>nom, max</sub>	[mm]	100	120	145	145	190	240	290	360
Min. substrate thickness	h <sub>min</sub>	[mm]	h <sub>ef</sub> + 30	h <sub>ef</sub> + 30	h <sub>ef</sub> +	h <sub>ef</sub> +	h <sub>ef</sub> +	h <sub>ef</sub> +	h <sub>ef</sub> +	h <sub>ef</sub> +
Min. spacing	s <sub>min</sub>	[mm]	0.5 * h <sub>ef</sub>	0.5 * h <sub>ef</sub>	0.5 * h <sub>ef</sub>	0.5 * h <sub>ef</sub>	0.5 * h <sub>ef</sub>	0.5 * h <sub>ef</sub>	0.5 * h <sub>ef</sub>	0.5 * h <sub>ef</sub>
Min. edge distance	c <sub>min</sub>	[mm]	0.5 * h <sub>ef</sub>	0.5 * h <sub>ef</sub>	0.5 * h <sub>ef</sub>	0.5 * h <sub>ef</sub>	0.5 * h <sub>ef</sub>	0.5 * h <sub>ef</sub>	0.5 * h <sub>ef</sub>	0.5 * h <sub>ef</sub>

## Installation data

Minimum working and curing time

Resin temperature	Concrete temperature	Curing time*	Working time
[°C]	[°C]	[min]	[min]
5	5	960	180
10	10	600	120
15	15	300	60
20	20	270	50
25	25	240	40
25	30	180	20

## Mechanical properties

REBARS AS ANCHORS

Size			Ø8	Ø10	Ø12	Ø14	Ø16	Ø20	Ø25	Ø32
<b>18G2</b>										
Nominal ultimate tensile strength - tension	$f_{uk}$	[N/mm <sup>2</sup> ]	480	480	480	480	480	480	480	480
Nominal yield strength - tension	$f_{yk}$	[N/mm <sup>2</sup> ]	355	355	355	355	355	355	355	355
Cross sectional area - tension	$A_s$	[mm <sup>2</sup> ]	50.3	78.5	113.1	153.9	201.1	314.2	490.9	804.2
Elastic section modulus	$W_{el}$	[mm <sup>3</sup> ]	50.3	98.2	169.6	269.4	402.1	785.4	1534	3217
Characteristic bending resistance	$M_{Rk,s}^0$	[Nm]	29	57	98	155	232	452	884	1853
Design bending resistance	M	[Nm]	19	38	65	103	154	302	589	1235
Allowable bending resistance	$M_{rec}$	[Nm]	14	27	47	74	110	215	421	882
<b>34G5</b>										
Nominal ultimate tensile strength - tension	$f_{uk}$	[N/mm <sup>2</sup> ]	500	500	500	500	500	500	500	500
Nominal yield strength - tension	$f_{yk}$	[N/mm <sup>2</sup> ]	410	410	410	410	410	410	410	410
Cross sectional area - tension	$A_s$	[mm <sup>2</sup> ]	50.3	78.5	113.1	153.9	201.1	314.2	490.9	804.2
Elastic section modulus	$W_{el}$	[mm <sup>3</sup> ]	50.3	98.2	169.6	269.4	402.1	785.4	1534	3217
Characteristic bending resistance	$M_{Rk,s}^0$	[Nm]	30	59	102	162	241	471	920	1930
Design bending resistance	M	[Nm]	20	39	68	108	161	314	614	1287
Allowable bending resistance	$M_{rec}$	[Nm]	14	28	48	77	115	224	438	919
<b>B500SP</b>										
Nominal ultimate tensile strength - tension	$f_{uk}$	[N/mm <sup>2</sup> ]	575	575	575	575	575	575	575	575
Nominal yield strength - tension	$f_{yk}$	[N/mm <sup>2</sup> ]	500	500	500	500	500	500	500	500
Cross sectional area - tension	$A_s$	[mm <sup>2</sup> ]	50.3	78.5	113.1	153.9	201.1	314.2	490.9	804.2
Elastic section modulus	$W_{el}$	[mm <sup>3</sup> ]	50.3	98.2	169.6	269.4	402.1	785.4	1534	3217
Characteristic bending resistance	$M_{Rk,s}^0$	[Nm]	35	68	117	186	277	542	1059	2220
Design bending resistance	M	[Nm]	23	45	78	124	185	361	706	1480
Allowable bending resistance	$M_{rec}$	[Nm]	17	32	56	89	132	258	504	1057
<b>RB500/BSt500S</b>										
Nominal ultimate tensile strength - tension	$f_{uk}$	[N/mm <sup>2</sup> ]	550	550	550	550	550	550	550	550
Nominal yield strength - tension	$f_{yk}$	[N/mm <sup>2</sup> ]	500	500	500	500	500	500	500	500
Cross sectional area - tension	$A_s$	[mm <sup>2</sup> ]	50.3	78.5	113.1	153.9	201.1	314.2	490.9	804.2
Elastic section modulus	$W_{el}$	[mm <sup>3</sup> ]	50.3	98.2	169.6	269.4	402.1	785.4	1534	3217
Characteristic bending resistance	$M_{Rk,s}^0$	[Nm]	33	65	112	178	265	518	1012	2123
Design bending resistance	M	[Nm]	22	43	75	119	177	346	675	1415
Allowable bending resistance	$M_{rec}$	[Nm]	16	31	53	85	126	247	482	1011

## Basic performance data

### REBARS AS ANCHORS

Size	Ø8	Ø10	Ø12	Ø14	Ø16	Ø20	Ø25	Ø32		
Substrate	Non-cracked concrete									
<b>CHARACTERISTIC LOAD</b>										
TENSION LOAD $N_{Rk}$										
A-II (e.g. 18G2)										
Minimum embedment depth	[kN]	16.6	26.4	36.1	35.2	50.5	66.4	83.7	107.0	-
Standard embedment depth	[kN]	22.1	33.9	49.8	48.4	70.6	111.9	153.7	187.8	-
Maximum embedment depth	[kN]	24.1	37.7	54.3	63.8	96.5	150.8	216.4	307.6	-
A-III (e.g. 34GS)										
Minimum embedment depth	[kN]	16.6	26.4	36.1	35.2	50.5	66.4	83.7	107.0	-
Standard embedment depth	[kN]	22.1	33.9	49.8	48.4	70.6	111.9	153.7	187.8	-
Maximum embedment depth	[kN]	25.1	39.3	56.5	63.8	100.5	157.1	216.4	307.6	-
A-III-N (e.g. RB500, BST500S, B500SP)										
Minimum embedment depth	[kN]	16.6	26.4	36.1	35.2	50.5	66.4	83.7	107.0	-
Standard embedment depth	[kN]	22.1	33.9	49.8	48.4	70.6	111.9	153.7	187.8	-
Maximum embedment depth	[kN]	27.6	43.2	62.2	63.8	110.6	172.8	216.4	307.6	-
SHEAR LOAD $V_{Rk}$										
A-II (e.g. 18G2)	[kN]	12.1	18.8	27.1	36.9	48.3	75.4	117.8	193.0	-
A-III (e.g. 34GS)	[kN]	12.6	19.6	28.3	38.5	50.3	78.5	122.7	201.1	-
A-III-N (e.g. RB500, BST500S, B500SP)	[kN]	13.8	21.6	31.1	42.3	55.3	86.4	135.0	221.2	-
<b>DESIGN LOAD</b>										
TENSION LOAD $N_{Rd}$										
A-II (e.g. 18G2)										
Minimum embedment depth	[kN]	9.20	14.7	20.1	19.5	28.1	36.9	46.5	59.5	-
Standard embedment depth	[kN]	12.3	18.8	27.6	26.9	39.2	62.2	85.4	104.3	-
Maximum embedment depth	[kN]	15.4	25.1	36.2	35.4	63.7	100.5	120.2	170.9	-
A-III (e.g. 34GS)										
Minimum embedment depth	[kN]	9.20	14.7	20.1	19.5	28.1	36.9	46.5	59.5	-
Standard embedment depth	[kN]	12.3	18.8	27.6	26.9	39.2	62.2	85.4	104.3	-
Maximum embedment depth	[kN]	15.4	25.1	36.4	35.4	63.7	100.5	120.2	170.9	-
A-III-N (e.g. RB500, BST500S, B500SP)										
Minimum embedment depth	[kN]	9.20	14.7	20.1	19.5	28.1	36.9	46.5	59.5	-
Standard embedment depth	[kN]	12.3	18.8	27.6	26.9	39.2	62.2	85.4	104.3	-
Maximum embedment depth	[kN]	15.4	25.1	36.4	35.4	63.7	100.5	120.2	170.9	-
SHEAR LOAD $V_{Rd}$										
A-II (e.g. 18G2)	[kN]	8.00	12.6	18.1	24.6	32.2	50.3	78.5	128.7	-
A-III (e.g. 34GS)	[kN]	8.40	13.1	18.8	25.7	33.5	52.4	81.8	134.0	-
A-III-N (e.g. RB500, BST500S, B500SP)	[kN]	9.20	14.4	20.7	28.2	36.9	57.6	90.0	147.4	-

## Basic performance data

## Design performance data

REBARS AS ANCHORS Minimum embedment depth

Size			Ø8	Ø10	Ø12	Ø14	Ø16	Ø20	Ø25	Ø32
Effective embedment depth	$h_{ef}$	[mm]	60.0	70.0	80.0	80.0	100.0	120.0	140.0	165.0
Spacing	$s_{cr,N}$	[mm]	180.0	210.0	240.0	240.0	300.0	360.0	420.0	495.0
Edge distance	$c_{cr,N}$	[mm]	90.0	105.0	120.0	120.0	150.0	180.0	210.0	248.0
<b>TENSION LOAD</b>										
<b>STEEL FAILURE; A-II (E.G. 18G2)</b>										
Characteristic resistance	$N_{Rk,s}$	[kN]	24.1	37.7	54.3	73.9	96.5	150.8	235.6	386.0
Design resistance $V_{Ms} = 1.5$	$N_{Rd,s}$	[kN]	16.1	25.1	36.2	49.3	64.3	100.5	157.1	257.4
<b>STEEL FAILURE; A-III (E.G. 34GS)</b>										
Characteristic resistance	$N_{Rk,s}$	[kN]	25.1	39.3	56.5	77.0	100.5	157.1	245.4	402.1
Design resistance $V_{Ms} = 1.5$	$N_{Rd,s}$	[kN]	16.8	26.2	37.7	51.3	67.0	104.7	163.6	268.1
<b>STEEL FAILURE; A-III-N (E.G. RB500, BST500S, B500SP)</b>										
Characteristic resistance	$N_{Rk,s}$	[kN]	27.6	43.2	62.2	84.7	110.6	172.8	270.0	442.3
Design resistance $V_{Ms} = 1.4$	$N_{Rd,s}$	[kN]	19.7	30.9	44.4	60.5	79.0	123.4	192.8	316.0
<b>PULL-OUT FAILURE; NON-CRACKED CONCRETE C20/25 (40°C/24°C)</b>										
Characteristic resistance	$N_{Rk,p}$	[kN]	16.6	26.4	-	35.2	-	-	-	-
Design resistance $V_{Mp} = 1.8$	$N_{Rd,p}$	[kN]	9.20	14.7	-	19.5	-	-	-	-
<b>PULL-OUT FAILURE; NON-CRACKED CONCRETE C20/25 (80°C/50°C)</b>										
Characteristic resistance	$N_{Rk,p}$	[kN]	15.1	24.2	33.2	31.7	-	-	-	-
Design resistance $V_{Mp} = 1.8$	$N_{Rd,p}$	[kN]	8.40	13.4	18.4	17.6	-	-	-	-
Increasing factors for $N_{Rd,p}$ - C30/37	$\psi_c$	-	1.04	1.04	1.04	1.04	1.04	1.04	1.04	1.04
Increasing factors for $N_{Rd,p}$ - C40/50	$\psi_c$	-	1.07	1.07	1.07	1.07	1.07	1.07	1.07	1.07
Increasing factors for $N_{Rd,p}$ - C50/60	$\psi_c$	-	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
<b>CONCRETE CONE FAILURE; NON-CRACKED CONCRETE C20/25 (40°C/24°C)</b>										
Characteristic resistance	$N_{Rk,c}$	[kN]	-	-	36.1	-	50.5	66.4	83.7	107.0
Design resistance $V_{Mc} = 1.8$	$N_{Rd,c}$	[kN]	-	-	20.1	-	28.1	36.9	46.5	59.5
<b>PULL-OUT FAILURE</b>										
Characteristic resistance	$N_{Rk,p}$	-	-	-	-	-	-	-	-	-
Design resistance	$N_{Rd,p}$	-	-	-	-	-	-	-	-	-
<b>CONCRETE CONE FAILURE; NON-CRACKED CONCRETE C20/25 (80°C/50°C)</b>										
Characteristic resistance	$N_{Rk,c}$	[kN]	-	-	-	-	50.5	66.4	83.7	107.0
Design resistance $V_{Mc} = 1.8$	$N_{Rd,c}$	[kN]	-	-	-	-	28.1	36.9	46.5	59.5
<b>EDGE FAILURE</b>										
<b>CONCRETE EDGE FAILURE; NON-CRACKED CONCRETE C20/25</b>										
Edge distance	$C_{min}$	[mm]	40.0	40.0	40.0	40.0	50.0	60.0	70.0	83.0
Characteristic resistance for $C_{min}$	$V_{Rk,c}$	[kN]	5.27	5.68	6.09	6.29	9.06	12.5	16.6	22.6
Design resistance $V_{Mc} = 1.5$	$V_{Rd,c}$	[kN]	3.51	3.79	4.06	4.19	6.04	8.34	11.0	15.0
<b>STEEL FAILURE; A-II (E.G. 18G2)</b>										
Characteristic resistance without lever arm	$V_{Rk,s}$	[kN]	12.1	18.8	27.1	36.9	48.3	75.4	117.8	193.0
Design resistance $V_{Ms} = 1.5$	$V_{Rd,s}$	[kN]	8.00	12.6	18.1	24.6	32.2	50.3	78.5	128.7
<b>STEEL FAILURE; A-III (E.G. 34GS)</b>										
Characteristic resistance without lever arm	$V_{Rk,s}$	[kN]	12.6	19.6	28.3	38.5	50.3	78.5	122.7	201.1
Design resistance $V_{Ms} = 1.5$	$V_{Rd,s}$	[kN]	8.40	13.1	18.8	25.7	33.5	52.4	81.8	134.0
<b>STEEL FAILURE; A-III-N (E.G. RB500, BST500S, B500SP)</b>										
Characteristic resistance without lever arm	$V_{Rk,s}$	[kN]	13.8	21.6	31.1	42.3	55.3	86.4	135.0	221.2
Design resistance $V_{Ms} = 1.5$	$V_{Rd,s}$	[kN]	9.20	14.4	20.7	28.2	36.9	57.6	90.0	147.4
<b>EDGE FAILURE</b>										
<b>CONCRETE EDGE FAILURE; NON-CRACKED CONCRETE C20/25</b>										
Edge distance	$C_{min}$	[mm]	40.0	40.0	40.0	40.0	50.0	60.0	70.0	83.0
Characteristic resistance for $C_{min}$	$V_{Rk,c}$	[kN]	5.27	5.68	6.09	6.29	9.06	12.5	16.6	22.6
Design resistance $V_{Mc} = 1.5$	$V_{Rd,c}$	[kN]	3.51	3.79	4.06	4.19	6.04	8.34	11.0	15.0

## Design performance data

Reduction / increasing resistance factors for edge distance and spacing

Edge distance (tension)

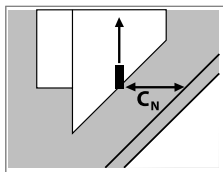


Table only valid for one edge  $<C_{cr,N}$  and  $S \geq S_{cr,N}$  For other cases use the Rawlplug Anchor Calculator

Reduction factors for edge distance  $<C_{cr,N}$  applicable to  $N_{Rd}$  or  $N_{rec}$  for cracked concrete from 'Basic Performance' table

$c_N$ [mm]	Ø8		Ø10		Ø12		Ø14		Ø16		Ø20		Ø25	Ø32
	$h \geq 1.13h_{min}$	$h_{min}$	$h \geq 1.25h_{min}$	$h_{min}$	$h \geq 1.24h_{min}$	$h_{min}$	$h \geq 1.24h_{min}$	$h_{min}$	$h \geq 1.24h_{min}$	$h_{min}$	$h \geq 1.25h_{min}$	$h_{min}$		
40	0.60	0.56	0.56	0.52	0.53	0.50	0.53	0.50						
50	0.67	0.61	0.62	0.57	0.58	0.54	0.58	0.54	0.53	0.50				
60	0.75	0.68	0.68	0.62	0.64	0.58	0.64	0.58	0.57	0.53	0.53	0.50		
70	0.83	0.74	0.75	0.68	0.69	0.63	0.69	0.63	0.62	0.57	0.57	0.53	0.53	
85	0.95	0.84	0.85	0.76	0.78	0.70	0.78	0.70	0.68	0.62	0.62	0.57	0.58	0.54
90	0.99	0.88	0.89	0.79	0.81	0.72	0.81	0.72	0.70	0.64	0.64	0.58	0.59	0.55
105	1.00	0.94	1.00	0.88	0.90	0.80	0.90	0.80	0.77	0.69	0.69	0.63	0.64	0.59
120		1.00		0.93	1.00	0.88	1.00	0.88	0.85	0.75	0.75	0.68	0.68	0.63
150				1.00		0.97		0.97	1.00	0.88	0.87	0.77	0.78	0.71
165						1.00		1.00		0.91	0.93	0.82	0.84	0.75
180										0.95	1.00	0.88	0.89	0.79
210										1.00		0.94	1.00	0.88
250												1.00		1.00

Edge distance (tension)

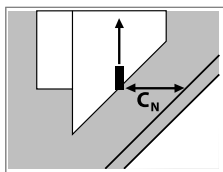


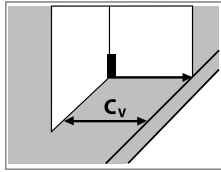
Table only valid for one edge  $<C_{cr,N}$  and  $S \geq S_{cr,N}$  For other cases use the Rawlplug Anchor Calculator

Reduction factors for edge distance  $<C_{cr,N}$  applicable to  $N_{Rd}$  or  $N_{rec}$  for non-cracked concrete from 'Basic Performance' table

$c_N$ [mm]	Ø8		Ø10		Ø12		Ø14		Ø16		Ø20		Ø25	Ø32
	$h \geq 1.13h_{min}$	$h_{min}$	$h \geq 1.25h_{min}$	$h_{min}$	$h \geq 1.24h_{min}$	$h_{min}$	$h \geq 1.24h_{min}$	$h_{min}$	$h \geq 1.24h_{min}$	$h_{min}$	$h \geq 1.25h_{min}$	$h_{min}$		
40	0.60	0.56	0.56	0.52	0.53	0.50	0.53	0.50						
50	0.67	0.61	0.62	0.57	0.58	0.54	0.58	0.54	0.53	0.50				
60	0.75	0.68	0.68	0.62	0.64	0.58	0.64	0.58	0.57	0.53	0.53	0.50		
70	0.83	0.74	0.75	0.68	0.69	0.63	0.69	0.63	0.62	0.57	0.57	0.53	0.53	
85	0.95	0.84	0.85	0.76	0.78	0.70	0.78	0.70	0.68	0.62	0.62	0.57	0.58	0.54
90	0.99	0.88	0.89	0.79	0.81	0.72	0.81	0.72	0.70	0.64	0.64	0.58	0.59	0.55
105	1.00	0.94	1.00	0.88	0.90	0.80	0.90	0.80	0.77	0.69	0.69	0.63	0.64	0.59
120		1.00		0.93	1.00	0.88	1.00	0.88	0.85	0.75	0.75	0.68	0.68	0.63
150				1.00		0.97		0.97	1.00	0.88	0.87	0.77	0.78	0.71
165						1.00		1.00		0.91	0.93	0.82	0.84	0.75
180										0.95	1.00	0.88	0.89	0.79
210										1.00		0.94	1.00	0.88
250												1.00		1.00

## Design performance data

Edge distance (shear)



Tables only valid for one edge  $>c_{min}$  and  $s \geq 3c_v$  For other cases use the Rawlplug Anchor Calculator

Increasing factors for edge distance  $>c_{min}$  applicable to  $V_{Rd,c}$  for non-cracked concrete from Design Performance table

$C_v$ [mm]	Ø8		Ø10		Ø12		Ø14		Ø16		Ø20		Ø25		Ø32	
	$h \geq 1.5c_v$	$h_{min}$	$h \geq 1.5c_v$	$h_{min}$	$h \geq 1.5c_v$	$h_{min}$	$h \geq 1.5c_v$	$h_{min}$	$h \geq 1.5c_v$	$h_{min}$	$h \geq 1.5c_v$	$h_{min}$	$h \geq 1.5c_v$	$h_{min}$	$h \geq 1.5c_v$	$h_{min}$
40	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00								
50	1.40	1.40	1.40	1.40	1.40	1.40	1.40	1.40	1.00	1.00						
60	1.84	1.84	1.84	1.84	1.84	1.84	1.84	1.84	1.31	1.31	1.00	1.00				
70	2.32	2.26	2.32	2.26	2.32	2.32	2.32	2.32	1.66	1.66	1.26	1.26	1.00	1.00		
83	2.99	2.68	2.99	2.68	2.99	2.89	2.99	2.89	2.14	2.14	1.63	1.63	1.29	1.29	1.00	1.00
90		2.90	3.38	2.90	3.38	3.13	3.38	3.13	2.41	2.41	1.84	1.84	1.46	1.46	1.13	1.13
100			3.95	3.23	3.95	3.48	3.95	3.48	2.83	2.77	2.15	2.15	1.71	1.71	1.32	1.32
120				3.87	5.20	4.17	5.20	4.17	3.72	3.33	2.83	2.76	2.24	2.24	1.74	1.74
150						5.21	7.26	5.21	5.20	4.16	3.95	3.46	3.14	2.99	2.43	2.43
165									5.74	5.99	4.57	4.56	3.80	3.62	3.29	2.80
180									6.26	6.83	4.99	5.20	4.15	4.12	3.58	3.19
210									7.30		5.82	6.55	4.84	5.20	4.18	4.02
250											6.93	8.51	5.76	6.75	4.98	5.23
300												6.91	8.87	5.97	6.87	5.07
350												8.06		6.97	8.66	5.92
400														7.96	10.58	6.76
450														8.96		7.61
500																8.45
550																9.30
600																10.14

## Design performance data

### Spacing

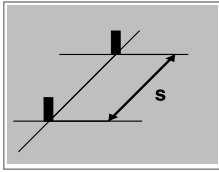


Table only valid for one spacing  $< s_{cr,N}$  and  $c \geq c_{cr,N}$ . For other cases use the Rawlplug Anchor Calculator

Reduction factors for spacing  $< S_{cr,N}$  applicable to  $N_{Rd}/V_{Rd}$  or  $N_{rec}/V_{rec}$  for non-cracked concrete from 'Basic Performance' table

s [mm]	Ø8		Ø10		Ø12		Ø14		Ø16		Ø20		Ø25	Ø32
	$h \geq 1.13h_{min}$	$h_{min}$	$h \geq 1.25h_{min}$	$h_{min}$	$h \geq 1.24h_{min}$	$h_{min}$	$h \geq 1.24h_{min}$	$h_{min}$	$h \geq 1.24h_{min}$	$h_{min}$	$h \geq 1.25h_{min}$	$h_{min}$		
40	0.61	0.58	0.60	0.57	0.58	0.56	0.58	0.56						
50	0.64	0.60	0.62	0.59	0.60	0.58	0.60	0.58	0.58	0.56				
60	0.67	0.63	0.64	0.61	0.63	0.59	0.63	0.59	0.60	0.58	0.58	0.56		
70	0.69	0.65	0.67	0.63	0.65	0.61	0.65	0.61	0.62	0.59	0.60	0.57	0.58	
85	0.74	0.68	0.70	0.65	0.68	0.63	0.68	0.63	0.64	0.61	0.62	0.59	0.60	0.59
100	0.78	0.71	0.74	0.68	0.71	0.66	0.71	0.66	0.67	0.63	0.64	0.60	0.62	0.60
125	0.85	0.76	0.80	0.72	0.76	0.70	0.76	0.70	0.71	0.66	0.67	0.63	0.65	0.63
150	0.92	0.81	0.86	0.77	0.81	0.73	0.81	0.73	0.75	0.69	0.71	0.66	0.68	0.65
180	0.99	0.88	0.93	0.82	0.88	0.78	0.88	0.78	0.80	0.73	0.75	0.69	0.71	0.68
200	1.00	0.92	0.98	0.86	0.92	0.81	0.92	0.81	0.83	0.75	0.78	0.71	0.74	0.70
225		0.97	1.00	0.90	0.97	0.85	0.97	0.85	0.88	0.78	0.81	0.73	0.77	0.73
250		1.00		0.95	1.00	0.89	1.00	0.89	0.92	0.81	0.85	0.76	0.80	0.75
275				0.99		0.93		0.93	0.96	0.84	0.88	0.79	0.83	0.78
300				1.00		0.97		0.97	1.00	0.88	0.92	0.81	0.86	0.80
325						1.00		1.00		0.91	0.95	0.84	0.89	0.83
360										0.95	1.00	0.88	0.93	0.86
400										1.00		0.92	0.98	0.90
440												0.96	1.00	0.94
500												1.00		1.00



## Design performance data

Standard embedment depth

Size			Ø8	Ø10	Ø12	Ø14	Ø16	Ø20	Ø25	Ø32
Effective embedment depth	$h_{ef}$	[mm]	80.0	90.0	110.0	110.0	125.0	170.0	210.0	240.0
Spacing	$s_{cr,N}$	[mm]	194.0	253.0	304.0	323.0	375.0	510.0	630.0	720.0
Edge distance	$c_{cr,N}$	[mm]	97.0	126.0	152.0	162.0	188.0	255.0	315.0	360.0
<b>TENSION LOAD</b>										
<b>STEEL FAILURE; A-II (E.G. 18G2)</b>										
Characteristic resistance	$N_{Rk,s}$	[kN]	24.1	37.7	54.3	73.9	96.5	150.8	235.6	386.0
Design resistance $V_{Ms} = 1.5$	$N_{Rd,s}$	[kN]	16.1	25.1	36.2	49.3	64.3	100.5	157.1	257.4
<b>STEEL FAILURE; A-III (E.G. 34GS)</b>										
Characteristic resistance	$N_{Rk,s}$	[kN]	25.1	39.3	56.5	77.0	100.5	157.1	245.4	402.1
Design resistance $V_{Ms} = 1.5$	$N_{Rd,s}$	[kN]	16.8	26.2	37.7	51.3	67.0	104.7	163.6	268.1
<b>STEEL FAILURE; A-III-N (E.G. RB500, BST500S, B500SP)</b>										
Characteristic resistance	$N_{Rk,s}$	[kN]	27.6	43.2	62.2	84.7	110.6	172.8	270.0	442.3
Design resistance $V_{Ms} = 1.4$	$N_{Rd,s}$	[kN]	19.7	30.9	44.4	60.5	79.0	123.4	192.8	316.0
<b>PULL-OUT FAILURE; NON-CRACKED CONCRETE C20/25 (40°C/24°C)</b>										
Characteristic resistance	$N_{Rk,p}$	[kN]	22.1	33.9	49.8	48.4	-	-	-	-
Design resistance $V_{Mp} = 1.8$	$N_{Rd,p}$	[kN]	12.3	18.8	27.6	26.9	-	-	-	-
<b>PULL-OUT FAILURE; NON-CRACKED CONCRETE C20/25 (80°C/50°C)</b>										
Characteristic resistance	$N_{Rk,p}$	[kN]	20.1	31.1	45.6	43.5	69.1	-	180.2	333.5
Design resistance $V_{Mp} = 1.8$	$N_{Rd,p}$	[kN]	11.2	17.3	25.3	24.2	38.4	-	77.9	100.5
Increasing Factors for $N_{Rd,p}$ - C30/37	$\Psi_c$	-	1.04	1.04	1.04	1.04	1.04	1.04	1.04	1.04
Increasing Factors for $N_{Rd,p}$ - C40/50	$\Psi_c$	-	1.07	1.07	1.07	1.07	1.07	1.07	1.07	1.07
Increasing Factors for $N_{Rd,p}$ - C50/60	$\Psi_c$	-	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
<b>CONCRETE CONE FAILURE; NON-CRACKED CONCRETE C20/25 (40°C/24°C)</b>										
Characteristic resistance	$N_{Rk,c}$	[kN]	-	-	-	-	70.6	111.9	153.7	187.8
Design resistance $V_{Mc} = 1.8$	$N_{Rd,c}$	[kN]	-	-	-	-	39.2	62.2	85.4	104.3
<b>PULL-OUT FAILURE</b>										
Characteristic resistance	$N_{Rk,p}$	-	-	-	-	-	-	-	-	-
Design resistance	$N_{Rd,p}$	-	-	-	-	-	-	-	-	-
<b>CONCRETE CONE FAILURE; NON-CRACKED CONCRETE C20/25 (80°C/50°C)</b>										
Characteristic resistance	$N_{Rk,c}$	[kN]	-	-	-	-	-	111.9	-	-
Design resistance $V_{Mc} = 1.8$	$N_{Rd,c}$	[kN]	-	-	-	-	-	62.2	-	-
<b>SHEAR LOAD</b>										
<b>CONCRETE EDGE FAILURE; NON-CRACKED CONCRETE C20/25</b>										
Edge distance	$C_{min}$	[mm]	40.0	45.0	55.0	55.0	63.0	85.0	105.0	120.0
Characteristic resistance for $C_{min}$	$V_{Rk,c}$	[kN]	5.60	7.00	9.80	10.2	12.8	21.1	30.4	39.2
Design resistance $V_{Mc} = 1.5$	$V_{Rd,c}$	[kN]	3.70	4.70	6.60	6.80	8.50	14.1	20.3	26.1
<b>STEEL FAILURE; A-II (E.G. 18G2)</b>										
Characteristic resistance without lever arm	$V_{Rk,s}$	[kN]	12.1	18.8	27.1	36.9	48.3	75.4	117.8	193.0
Design resistance $V_{Ms} = 1.5$	$V_{Rd,s}$	[kN]	8.00	12.6	18.1	24.6	32.2	50.3	78.5	128.7
<b>STEEL FAILURE; A-III (E.G. 34GS)</b>										
Characteristic resistance without lever arm	$V_{Rk,s}$	[kN]	12.6	19.6	28.3	38.5	50.3	78.5	122.7	201.1
Design resistance $V_{Ms} = 1.5$	$V_{Rd,s}$	[kN]	8.40	13.1	18.8	25.7	33.5	52.4	81.8	134.0
<b>STEEL FAILURE; A-III-N (E.G. RB500, BST500S, B500SP)</b>										
Characteristic resistance without lever arm	$V_{Rk,s}$	[kN]	13.8	21.6	31.1	42.3	55.3	86.4	135.0	221.2
Design resistance $V_{Ms} = 1.5$	$V_{Rd,s}$	[kN]	9.20	14.4	20.7	28.2	36.9	57.6	90.0	147.4

## Design performance data

### Reduction / increasing resistance factors for edge distance and spacing

Edge distance (tension)

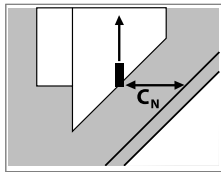


Table only valid for one edge  $<C_{cr,N}$  and  $S \geq S_{cr,N}$ . For other cases use the Rawlplug Anchor Calculator

Reduction factors for edge distance  $<C_{cr,N}$  applicable to  $N_{Rd}$  or  $N_{rec}$  for cracked concrete from 'Basic Performance' table

$c_N$ [mm]	Ø8		Ø10		Ø12		Ø14		Ø16		Ø20		Ø25		Ø32	
	$h \geq 1.28h_{min}$	$h_{min}$	$h \geq 1.31h_{min}$	$h_{min}$	$h \geq 1.31h_{min}$	$h_{min}$	$h \geq 1.31h_{min}$	$h_{min}$	$h \geq 1.30h_{min}$	$h_{min}$	$h \geq 1.33h_{min}$	$h_{min}$	$h \geq 1.33h_{min}$	$h_{min}$	$h \geq 1.31h_{min}$	$h_{min}$
40	0.58	0.51														
45	0.61	0.54	0.55	0.50												
55	0.68	0.58	0.60	0.54	0.55	0.51	0.54	0.50								
63	0.74	0.62	0.64	0.57	0.58	0.53	0.57	0.53	0.53	0.50						
85	0.90	0.74	0.75	0.66	0.68	0.60	0.65	0.59	0.61	0.56	0.53	0.50				
105	1.00	0.83	0.87	0.75	0.77	0.67	0.74	0.66	0.68	0.62	0.58	0.54	0.56	0.54		
120		0.88	0.96	0.82	0.84	0.72	0.80	0.71	0.73	0.66	0.62	0.57	0.59	0.57	0.54	0.54
140		0.94	1.00	0.89	0.94	0.80	0.90	0.79	0.81	0.72	0.67	0.61	0.64	0.61	0.58	0.57
165		1.00		0.96	1.00	0.88	1.00	0.88	0.91	0.80	0.74	0.67	0.69	0.67	0.63	0.62
180				1.00		0.91		0.91	0.97	0.85	0.78	0.70	0.73	0.70	0.66	0.64
200						0.95		0.95	1.00	0.90	0.84	0.74	0.78	0.75	0.70	0.68
225						1.00		1.00		0.95	0.91	0.80	0.85	0.81	0.75	0.73
265										1.00	1.00	0.89	0.95	0.90	0.83	0.81
300												0.94	1.00	0.98	0.91	0.88
320												0.97		1.00	0.95	0.93
360												1.00			1.00	1.00

Edge distance (tension)

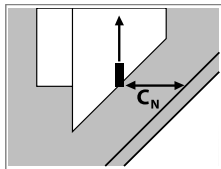


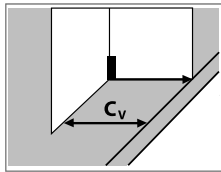
Table only valid for one edge  $<C_{cr,N}$  and  $S \geq S_{cr,N}$ . For other cases use the Rawlplug Anchor Calculator

Reduction factors for edge distance  $<C_{cr,N}$  applicable to  $N_{Rd}$  or  $N_{rec}$  for non-cracked concrete from 'Basic Performance' table

$c_N$ [mm]	Ø8		Ø10		Ø12		Ø14		Ø16		Ø20		Ø25		Ø32	
	$h \geq 1.28h_{min}$	$h_{min}$	$h \geq 1.31h_{min}$	$h_{min}$	$h \geq 1.31h_{min}$	$h_{min}$	$h \geq 1.31h_{min}$	$h_{min}$	$h \geq 1.30h_{min}$	$h_{min}$	$h \geq 1.33h_{min}$	$h_{min}$	$h \geq 1.33h_{min}$	$h_{min}$	$h \geq 1.31h_{min}$	$h_{min}$
40	0.58	0.51														
45	0.61	0.54	0.55	0.50												
55	0.68	0.58	0.60	0.54	0.55	0.51	0.54	0.50								
63	0.74	0.62	0.64	0.57	0.58	0.53	0.57	0.53	0.53	0.50						
85	0.90	0.74	0.75	0.66	0.68	0.60	0.65	0.59	0.61	0.56	0.53	0.50				
105	1.00	0.83	0.87	0.75	0.77	0.67	0.74	0.66	0.68	0.62	0.58	0.54	0.56	0.54		
120		0.88	0.96	0.82	0.84	0.72	0.80	0.71	0.73	0.66	0.62	0.57	0.59	0.57	0.54	0.54
140		0.94	1.00	0.89	0.94	0.80	0.90	0.79	0.81	0.72	0.67	0.61	0.64	0.61	0.58	0.57
165		1.00		0.96	1.00	0.88	1.00	0.88	0.91	0.80	0.74	0.67	0.69	0.67	0.63	0.62
180				1.00		0.91		0.91	0.97	0.85	0.78	0.70	0.73	0.70	0.66	0.64
200						0.95		0.95	1.00	0.90	0.84	0.74	0.78	0.75	0.70	0.68
225						1.00		1.00		0.95	0.91	0.80	0.85	0.81	0.75	0.73
265										1.00	1.00	0.89	0.95	0.90	0.83	0.81
300												0.94	1.00	0.98	0.91	0.88
320												0.97		1.00	0.95	0.93
360												1.00			1.00	1.00

## Design performance data

Edge distance (shear)



Tables only valid for one edge  $>c_{min}$  and  $s \geq 3c_v$  For other cases use the Rawplug Anchor Calculator

Increasing factors for edge distance  $>C_{min}$  applicable to  $V_{Rd,c}$  for non-cracked concrete from Design Performance table

$C_v$ [mm]	Ø8		Ø10		Ø12		Ø14		Ø16		Ø20		Ø25		Ø32	
	$h \geq 1.5c_v$	$h_{min}$	$h \geq 1.5c_v$	$h_{min}$	$h \geq 1.5c_v$	$h_{min}$	$h \geq 1.5c_v$	$h_{min}$	$h \geq 1.5c_v$	$h_{min}$	$h \geq 1.5c_v$	$h_{min}$	$h \geq 1.5c_v$	$h_{min}$	$h \geq 1.5c_v$	$h_{min}$
40	1.00	1.00														
45	1.19	1.19	1.00	1.00												
55	1.61	1.61	1.35	1.35	1.00	1.00	1.00	1.00								
63	1.98	1.98	1.66	1.66	1.23	1.23	1.23	1.23	1.00	1.00						
85	3.10	2.88	2.60	2.52	1.92	1.92	1.92	1.92	1.57	1.57	1.00	1.00				
105		3.55	3.56	3.11	2.64	2.54	2.64	2.54	2.15	2.15	1.37	1.37	1.00	1.00		
120				3.56	3.22	2.90	3.22	2.90	2.63	2.55	1.68	1.68	1.22	1.22	1.00	1.00
150					4.50	3.63	4.50	3.63	3.67	3.18	2.34	2.33	1.71	1.71	1.40	1.40
180						4.35		4.35	4.83	3.82	3.08	2.79	2.24	2.24	1.84	1.84
225										4.78	4.31	3.49	3.14	2.83	2.57	2.50
250												3.88	3.67	3.14	3.01	2.78
300												4.66	4.83	3.77	3.95	3.33
350														4.40	4.98	3.89
400														5.02	6.09	4.44
450																5.00
500																5.56
550																6.11

## Design performance data

### Spacing

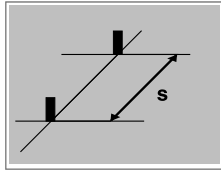


Table only valid for one spacing  $< s_{cr,N}$  and  $c \geq c_{cr,N}$ . For other cases use the Rawlplug Anchor Calculator

Reduction factors for spacing  $< s_{cr,N}$  applicable to  $N_{Rd}/V_{Rd}$  or  $N_{rec}/V_{rec}$  for non-cracked concrete from 'Basic Performance' table

s [mm]	Ø8		Ø10		Ø12		Ø14		Ø16		Ø20		Ø25		Ø32	
	$h \geq 1.28h_{min}$	$h_{min}$	$h \geq 1.31h_{min}$	$h_{min}$	$h \geq 1.31h_{min}$	$h_{min}$	$h \geq 1.31h_{min}$	$h_{min}$	$h \geq 1.30h_{min}$	$h_{min}$	$h \geq 1.33h_{min}$	$h_{min}$	$h \geq 1.33h_{min}$	$h_{min}$	$h \geq 1.31h_{min}$	$h_{min}$
40	0.60	0.56														
45	0.62	0.57	0.59	0.56												
55	0.64	0.59	0.61	0.58	0.59	0.56	0.59	0.56								
63	0.66	0.60	0.62	0.59	0.60	0.57	0.60	0.57	0.58	0.56						
85	0.72	0.63	0.67	0.62	0.64	0.60	0.63	0.60	0.61	0.59	0.58	0.56				
105	0.77	0.66	0.71	0.65	0.67	0.62	0.66	0.62	0.64	0.61	0.60	0.58	0.59	0.58		
120	0.81	0.69	0.74	0.67	0.70	0.64	0.69	0.64	0.66	0.62	0.62	0.59	0.61	0.60	0.59	0.58
150	0.89	0.73	0.80	0.71	0.75	0.67	0.73	0.67	0.70	0.65	0.65	0.61	0.63	0.62	0.61	0.60
180	0.96	0.78	0.86	0.75	0.80	0.70	0.78	0.70	0.74	0.68	0.68	0.63	0.66	0.64	0.63	0.63
200	1.00	0.81	0.90	0.78	0.83	0.73	0.81	0.73	0.77	0.70	0.70	0.65	0.68	0.66	0.65	0.64
260		0.91	1.00	0.86	0.93	0.80	0.90	0.80	0.85	0.76	0.76	0.69	0.73	0.71	0.69	0.68
300		0.97		0.92	0.99	0.84	0.96	0.84	0.90	0.80	0.80	0.72	0.77	0.74	0.72	0.71
330		1.00		0.96	1.00	0.88	1.00	0.88	0.94	0.83	0.83	0.74	0.79	0.76	0.74	0.73
380				1.00		0.93		0.93	1.00	0.88	0.88	0.78	0.84	0.80	0.78	0.76
450						1.00		1.00		0.95	0.94	0.83	0.90	0.86	0.83	0.81
500										1.00	0.99	0.87	0.94	0.90	0.87	0.85
550											1.00	0.91	1.00	0.94	0.91	0.89
630												0.96		1.00	0.96	0.94
720												1.00			1.00	1.00

### Maximum embedment depth

Size		Ø8	Ø10	Ø12	Ø14	Ø16	Ø20	Ø25	Ø32
Effective embedment depth	$h_{ef}$ [mm]	100.0	120.0	145.0	145.0	190.0	240.0	290.0	360.0
Spacing	$s_{cr,N}$ [mm]	194.0	253.0	304.0	323.0	405.0	506.0	563.0	681.0
Edge distance	$c_{cr,N}$ [mm]	97.0	126.0	152.0	162.0	202.0	253.0	281.0	341.0
<b>TENSION LOAD</b>									
<b>STEEL FAILURE; A-II (E.G. 18G2)</b>									
Characteristic resistance	$N_{Rk,s}$ [kN]	24.1	37.7	54.3	73.9	96.5	150.8	235.6	386.0
Design resistance $V_{Ms} = 1.5$	$N_{Rd,s}$ [kN]	16.1	25.1	36.2	49.3	64.3	100.5	157.1	257.4
<b>STEEL FAILURE; A-III (E.G. 34GS)</b>									
Characteristic resistance	$N_{Rk,s}$ [kN]	25.1	39.3	56.5	77.0	100.5	157.1	245.4	402.1
Design resistance $V_{Ms} = 1.5$	$N_{Rd,s}$ [kN]	16.8	26.2	37.7	51.3	67.0	104.7	163.6	268.1
<b>STEEL FAILURE; A-III-N (E.G. RB500, BST500S, B500SP)</b>									
Characteristic resistance	$N_{Rk,s}$ [kN]	27.6	43.2	62.2	84.7	110.6	172.8	270.0	442.3
Design resistance $V_{Ms} = 1.4$	$N_{Rd,s}$ [kN]	19.7	30.9	44.4	60.5	79.0	123.4	192.8	316.0
<b>PULL-OUT FAILURE; NON-CRACKED CONCRETE C20/25 (40°C/24°C)</b>									
Characteristic resistance	$N_{Rk,p}$ [kN]	27.6	45.2	65.6	63.8	114.6	181.0	216.4	307.6
Design resistance $V_{Mp} = 1.8$	$N_{Rd,p}$ [kN]	15.4	25.1	36.4	35.4	63.7	100.5	120.2	170.9
<b>PULL-OUT FAILURE; NON-CRACKED CONCRETE C20/25 (80°C/50°C)</b>									
Characteristic resistance	$N_{Rk,p}$ [kN]	25.1	41.5	60.1	57.4	105.1	165.9	193.6	271.4
Design resistance $V_{Mp} = 1.8$	$N_{Rd,p}$ [kN]	14.0	23.0	33.4	31.9	58.4	92.2	107.6	150.8
Increasing factors for $N_{Rd,p}$ - C30/37	$\psi_c$	-	1.04	1.04	1.04	1.04	1.04	1.04	1.04
Increasing factors for $N_{Rd,p}$ - C40/50	$\psi_c$	-	1.07	1.07	1.07	1.07	1.07	1.07	1.07
Increasing factors for $N_{Rd,p}$ - C50/60	$\psi_c$	-	1.09	1.09	1.09	1.09	1.09	1.09	1.09

## Design performance data

Reduction / increasing resistance factors for edge distance and spacing

Edge distance (tension)

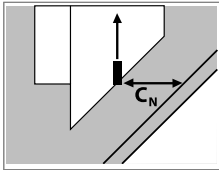


Table only valid for one edge  $< c_{cr,N}$  and  $S \geq S_{cr,N}$ . For other cases use the Rawlplug Anchor Calculator

Reduction factors for edge distance  $< c_{cr,N}$  applicable to  $N_{Rd}$  or  $N_{rec}$  for cracked concrete from 'Basic Performance' table

$c_N$ [mm]	Ø8		Ø10		Ø12		Ø14		Ø16		Ø20		Ø25		Ø32	
	$h \geq 1.33h_{min}$	$h_{min}$	$h \geq 1.37h_{min}$	$h_{min}$	$h \geq 1.37h_{min}$	$h_{min}$	$h \geq 1.37h_{min}$	$h_{min}$	$h \geq 1.38h_{min}$	$h_{min}$	$h \geq 1.39h_{min}$	$h_{min}$	$h \geq 1.39h_{min}$	$h_{min}$	$h \geq 1.39h_{min}$	$h_{min}$
50	0.65	0.53														
60	0.72	0.58	0.62	0.53												
73	0.81	0.63	0.69	0.57	0.63	0.53	0.61	0.52								
95	0.98	0.73	0.81	0.65	0.72	0.59	0.70	0.58	0.62	0.53						
100	1.00	0.75	0.84	0.66	0.74	0.60	0.72	0.60	0.63	0.54						
120		0.80	0.96	0.74	0.84	0.66	0.80	0.65	0.70	0.58	0.62	0.53				
140		0.85	1.00	0.79	0.94	0.72	0.90	0.71	0.77	0.62	0.67	0.56				
145		0.86		0.80	0.96	0.74	0.92	0.73	0.79	0.63	0.69	0.57	0.69	0.57		
165		0.91		0.84	1.00	0.78	1.00	0.78	0.86	0.68	0.74	0.60	0.73	0.60		
180		0.95		0.88		0.81		0.81	0.91	0.71	0.78	0.63	0.79	0.63	0.66	0.57
205		1.00		0.93		0.85		0.85	1.00	0.77	0.85	0.67	0.94	0.68	0.71	0.61
260				1.00		0.95		0.95		0.84	1.00	0.77	1.00	0.78	0.82	0.69
300						1.00		1.00		0.89		0.81		0.84	0.91	0.75
350										0.96		0.86		0.90	1.00	0.82
400										1.00		0.92		0.96		0.87
450												0.97		1.00		0.92
500												1.00				0.96
550																1.00

## Design performance data

Edge distance (tension)

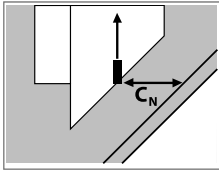


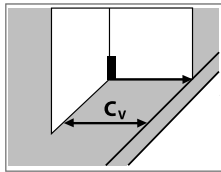
Table only valid for one edge  $< C_{cr,N}$  and  $S \geq S_{cr,N}$ . For other cases use the Rawlplug Anchor Calculator

Reduction factors for edge distance  $< C_{cr,N}$  applicable to  $N_{rd}$  or  $N_{rec}$  for non-cracked concrete from 'Basic Performance' table

$c_N$ [mm]	Ø8		Ø10		Ø12		Ø14		Ø16		Ø20		Ø25		Ø32	
	$h \geq 1.33h_{min}$	$h_{min}$	$h \geq 1.37h_{min}$	$h_{min}$	$h \geq 1.37h_{min}$	$h_{min}$	$h \geq 1.37h_{min}$	$h_{min}$	$h \geq 1.38h_{min}$	$h_{min}$	$h \geq 1.39h_{min}$	$h_{min}$	$h \geq 1.39h_{min}$	$h_{min}$	$h \geq 1.39h_{min}$	$h_{min}$
50	0.65	0.53														
60	0.72	0.58	0.62	0.53												
73	0.81	0.63	0.69	0.57	0.63	0.53	0.61	0.52								
95	0.98	0.73	0.81	0.65	0.72	0.59	0.70	0.58	0.62	0.53						
100	1.00	0.75	0.84	0.66	0.74	0.60	0.72	0.60	0.63	0.54						
120		0.80	0.96	0.74	0.84	0.66	0.80	0.65	0.70	0.58	0.62	0.53				
140		0.85	1.00	0.79	0.94	0.72	0.90	0.71	0.77	0.62	0.67	0.56				
145		0.86		0.80	0.96	0.74	0.92	0.73	0.79	0.63	0.69	0.57	0.69	0.57		
165		0.91		0.84	1.00	0.78	1.00	0.78	0.86	0.68	0.74	0.60	0.73	0.60		
180		0.95		0.88		0.81		0.81	0.91	0.71	0.78	0.63	0.79	0.63	0.66	0.57
205		1.00		0.93		0.85		0.85	1.00	0.77	0.85	0.67	0.94	0.68	0.71	0.61
260				1.00		0.95		0.95		0.84	1.00	0.77	1.00	0.78	0.82	0.69
300						1.00		1.00		0.89		0.81		0.84	0.91	0.75
350										0.96		0.86		0.90	1.00	0.82
400										1.00		0.92		0.96		0.87
450												0.97		1.00		0.92
500												1.00				0.96
550																1.00

## Design performance data

Edge distance (shear)



Tables only valid for one edge  
 $>c_{min}$  and  $s \geq 3c_v$  For other cases  
 use the Rawlplug Anchor Calculator

Increasing factors for edge distance  $>c_{min}$  applicable to  $V_{Rd,c}$  for non-cracked concrete from Design Performance table

$C_v$ [mm]	Ø8		Ø10		Ø12		Ø14		Ø16		Ø20		Ø25		Ø32	
	$h \geq 1.5c_v$	$h_{min}$	$h \geq 1.5c_v$	$h_{min}$	$h \geq 1.5c_v$	$h_{min}$	$h \geq 1.5c_v$	$h_{min}$	$h \geq 1.5c_v$	$h_{min}$	$h \geq 1.5c_v$	$h_{min}$	$h \geq 1.5c_v$	$h_{min}$	$h \geq 1.5c_v$	$h_{min}$
50	1.00	1.00														
60	1.31	1.31	1.00	1.00												
73	1.76	1.76	1.34	1.34	1.00	1.00	1.00	1.00								
95	2.62	2.50	1.99	1.99	1.48	1.48	1.48	1.48	1.00	1.00						
100		2.63	2.15	2.15	1.60	1.60	1.60	1.60	1.08	1.08						
105					1.73	1.73	1.73	1.73	1.16	1.16						
120					2.11	2.11	2.11	2.11	1.42	1.42	1.00	1.00				
145					2.80	2.55	2.80	2.55	1.89	1.89	1.33	1.33	1.00	1.00		
180							3.87	3.17	2.61	2.43	1.84	1.84	1.38	1.38	1.00	1.00
200										2.70	2.15	2.12	1.62	1.62	1.17	1.17
230											2.65	2.44	2.00	2.00	1.44	1.44
250												2.65	2.26	2.20	1.64	1.64
300													2.98	2.64	2.15	2.13
350														3.08	2.71	2.48
400															3.31	2.84
450																3.19

## Design performance data

### Spacing

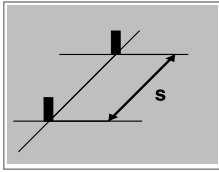


Table only valid for one spacing  $< s_{cr,N}$  and  $c \geq c_{cr,N}$ . For other cases use the Rawlplug Anchor Calculator

Reduction factors for spacing  $< s_{cr,N}$  applicable to  $N_{Rd}/V_{Rd}$  or  $N_{rec}/V_{rec}$  for non-cracked concrete from 'Basic Performance' table

s [mm]	Ø8		Ø10		Ø12		Ø14		Ø16		Ø20		Ø25		Ø32	
	$h \geq 1.33h_{min}$	$h_{min}$	$h \geq 1.37h_{min}$	$h_{min}$	$h \geq 1.37h_{min}$	$h_{min}$	$h \geq 1.37h_{min}$	$h_{min}$	$h \geq 1.38h_{min}$	$h_{min}$	$h \geq 1.39h_{min}$	$h_{min}$	$h \geq 1.39h_{min}$	$h_{min}$	$h \geq 1.39h_{min}$	$h_{min}$
50	0.63	0.56														
60	0.65	0.58	0.62	0.56												
73	0.69	0.59	0.64	0.58	0.62	0.56	0.61	0.56								
95	0.75	0.62	0.69	0.60	0.66	0.58	0.65	0.58	0.62	0.56						
100	0.76	0.63	0.70	0.60	0.66	0.59	0.65	0.59	0.62	0.57						
120	0.81	0.65	0.74	0.63	0.70	0.60	0.69	0.60	0.65	0.58	0.62	0.56				
145	0.87	0.68	0.79	0.65	0.74	0.63	0.72	0.63	0.68	0.60	0.64	0.58	0.63	0.58		
180	0.96	0.73	0.86	0.69	0.80	0.66	0.78	0.66	0.72	0.62	0.68	0.59	0.66	0.60	0.63	0.58
200	1.00	0.75	0.90	0.71	0.83	0.67	0.81	0.67	0.75	0.63	0.70	0.60	0.68	0.61	0.65	0.59
225		0.78	0.94	0.73	0.87	0.69	0.85	0.69	0.78	0.65	0.72	0.62	0.70	0.63	0.67	0.60
250		0.81	0.99	0.76	0.91	0.72	0.89	0.72	0.81	0.66	0.75	0.63	0.72	0.64	0.68	0.62
280		0.85	1.00	0.79	0.96	0.74	0.93	0.74	0.85	0.68	0.78	0.65	0.75	0.66	0.71	0.63
320		0.90		0.83	1.00	0.78	0.99	0.78	0.90	0.71	0.82	0.67	0.78	0.68	0.73	0.65
400		1.00		0.92		0.84	1.00	0.84	0.99	0.76	0.90	0.71	0.86	0.73	0.79	0.69
450				0.97		0.89		0.89	1.00	0.80	0.94	0.73	0.90	0.76	0.83	0.71
510				1.00		0.94		0.94		0.84	1.00	0.77	0.95	0.79	0.87	0.74
630						1.00		1.00		0.91		0.83	1.00	0.86	0.96	0.79
760										1.00		0.90		0.94	1.00	0.85
955												1.00		1.00	1.00	0.94
1000																0.96
1100																1.00

## Product commercial data

Size	Product Code	Volume [ml]	Quantity [pcs]			Weight [kg]			Bar Codes
			Box	Outer	Pallet	Box	Outer	Pallet	
Ø32	R-KEX-II-385 <sup>1)</sup>	385	10	10	380	6.7	6.7	285.0	5906675028538
	R-KEX-II-600 <sup>1)</sup>	600	7	7	441	7.0	7.0	472.7	5906675293721

<sup>1)</sup> ETA-13/0454